

## STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY GOVERNOR

LYNDO TIPPETT SECRETARY

July 14, 2003

## Addendum No. 1

RE:

Contract ID: C200692 (Proposal No.1)

Work Order No. 8.U401717 F. A. No. NHF-540 (1) Wake County (R-2000G)

I-540 (Northern Wake Expressway) From South of SR-2215 (Buffaloe Road) To US-64 East of Knightdale.

## July 22, 2003 Special Letting

To Whom It May Concern:

Reference is made to the plans and proposal form furnished to you on the abovementioned project.

The following revisions have been made to the plans:

Roadway typical sheet nos. 2-A and 2-B have been revised to correct the limits of concrete paving on the ramps. Please void sheet nos. 2-A and 2-B in your plans and staple the revised sheet nos. 2-A and 2-B thereto.

Sheet nos. S-35, S-37, S-69, and S-71 of the structure plans have been revised to show a work bridge instead of a workpad for temporary access. Please void sheet nos. S-35, S-37, S-69 and S-71 in your plans and staple the revised sheet nos. S-35, S-37, S-69 and S-71 thereto.

Sheet nos. S-105, S-107 and S-108 of the structure plans have been revised to show temporary shoring and to indicate no traffic on stage 1 bridge construction. Please void sheet nos. S-105, S-107 and S-108 in your plans and staple the revised sheet nos. S-105, S-107 and S-108 thereto.

The following revisions have been made to the proposal form:

Page No. 432 has been revised to show the location of wetland sites no. 13 and 14. Please void Page No. 432 in your proposal and staple the revised Page No. 432 thereto.

LOCATION:

New Page Nos. 432-A, 432-B and 432-C and are being added to include wetland sites No. 13 and 14. Please add new Page Nos. 432-A, 432-B and 432-C after revised Page No. 432. Page No. 473 has been revised to include sites 13 and 14. Please void Page no. 473 in your proposal and staple the revised Page No. 473 thereto.

Page No. 458, labeled "Workpad Detail" is obsolete. Please void Page No. 458 in your proposal and staple the revised Page No. 458 (Intentionally Blank) thereto.

By copy of this addendum, the quantities for the following pay items shown on Page No. 3 and 4 of the item sheets are hereby revised:

<u>Line Item &amp; Description</u> 51-1121000000-M-520 Aggregate Base Course	Old Quantity 38,100MTN	New Quantity 35,900MTN
56-1489000000-M-610 Asphalt Conc. Base Course, Type B25.0B	26,900MTN	26,925MTN
59-1503000000-M-610 Asphalt Conc. Inter. Course, Type I19.0C	20,100MTN	16,780MTN
60-1519000000-M-610 Asphalt Conc. Surface Course, Type S9.5B	11,775MTN	11,920MTN
61-1539000000-M-610 Asphalt Conc. Surface Course, Type S12.5C	12,750MTN	12,145MTN
62-1560000000-M-620 Asphalt Binder For Plant Mix, Grade PG-64-22	3,760MTN	3,694MTN
63-1565000000-M-620 Asphalt Binder For Plant Mix, Grade PG-70-22	705MTN	669MTN
65-187470000000-M-710 300MM Port. Cem. Conc. Pavement through Lanes (with Dowels)	131,090M2	131,500M2
66-18690000000-M-710 300MM Port. Cem. Conc. Pavement Miscellaneous (without Dowels)	6,810M2	4,350M2
68-1913000000-M-720 Concrete Shoulders Adjacent to 300MM Pavement	70,430M2	72,975M2

The Contractor's bid prices should be based on these revised pay item quantities. By copy of this addendum, the following lump sum pay item on Page 20 of the item sheets, is hereby deleted: "328-8017000000-N-SP Construction, Maintenance, & Removal of Temporary Access at Sta. 445 + 88.000-L-RT-". All cost for the temporary access at this location should be included in the pay item for the left lane. The contract will be prepared accordingly.

The Expedite file has been updated to reflect these revisions. Please download the EBS addendum file and follow the instructions for applying the addendum. Bid Express will not accept your bid unless all addendums have been applied.

Sincerely,

R. A. Garris, P.E. Contract Officer

## RAG/jag/pa

### Attachments

Cc: Mr. J. D. Goins, P.E.

Mr. S. D. DeWitt, P.E.

Mr. J. G. Nance, P.E.

Ms. D. M. Barbour, P.E.

Mr. J. V. Barbour, P.E.

Mr. Scott Blevins, P.E.

Mr. J. F. Sullivan, III (FHWA)

Mr. Mark Staley (2)

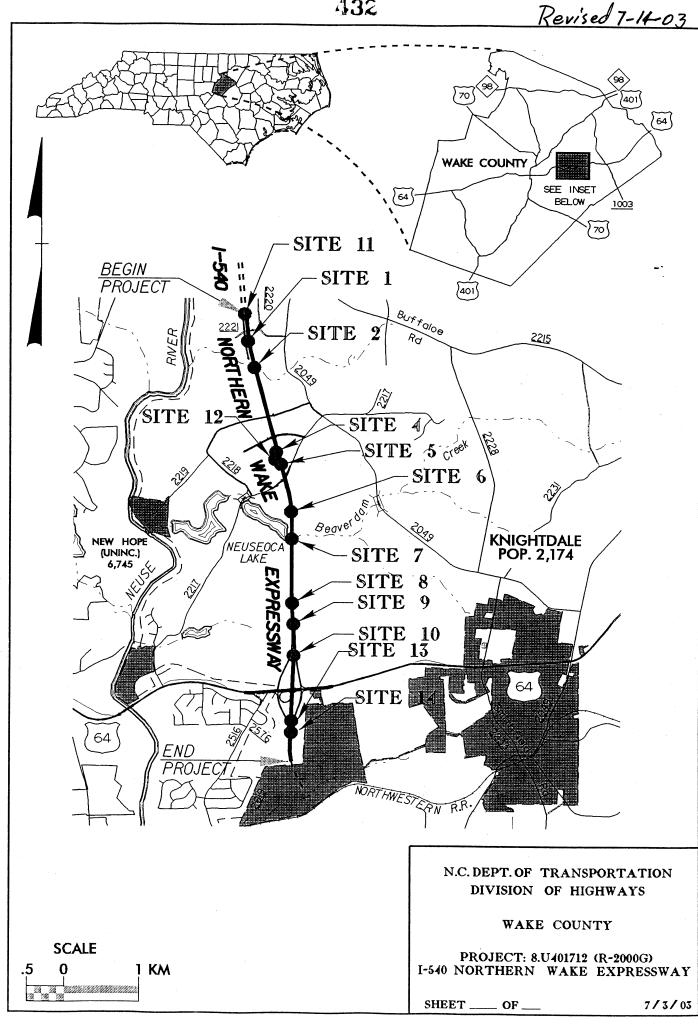
Mr. Aydren Flowers

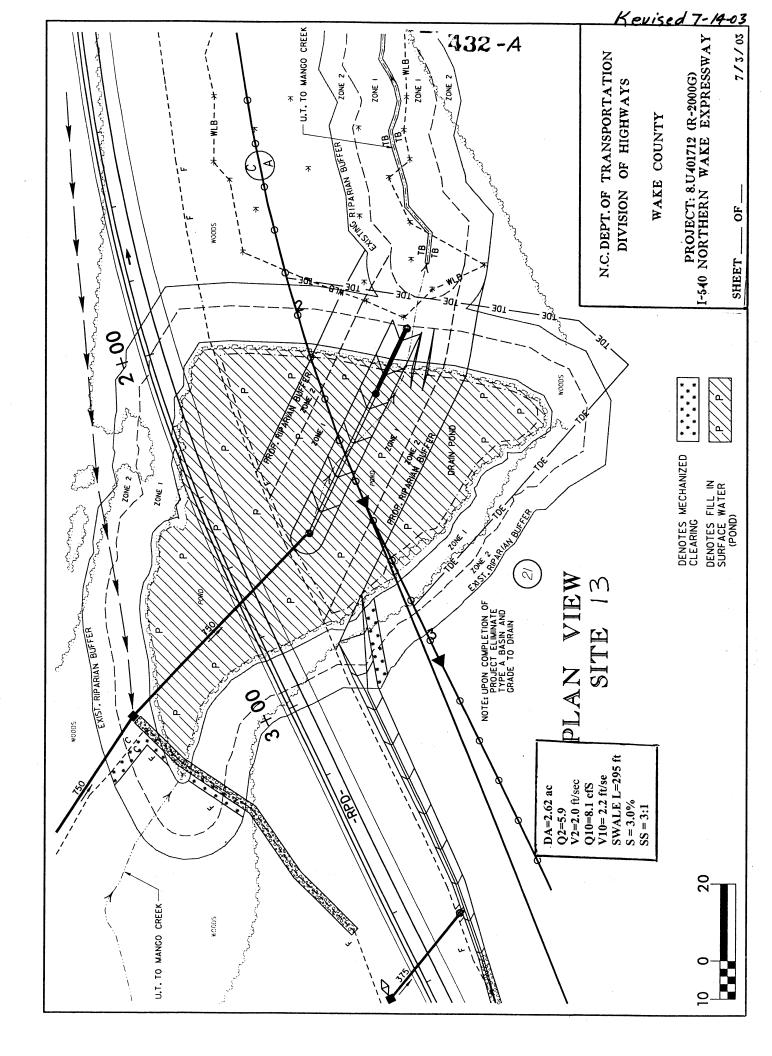
Mr. Ron Davenport, Jr., P.E.

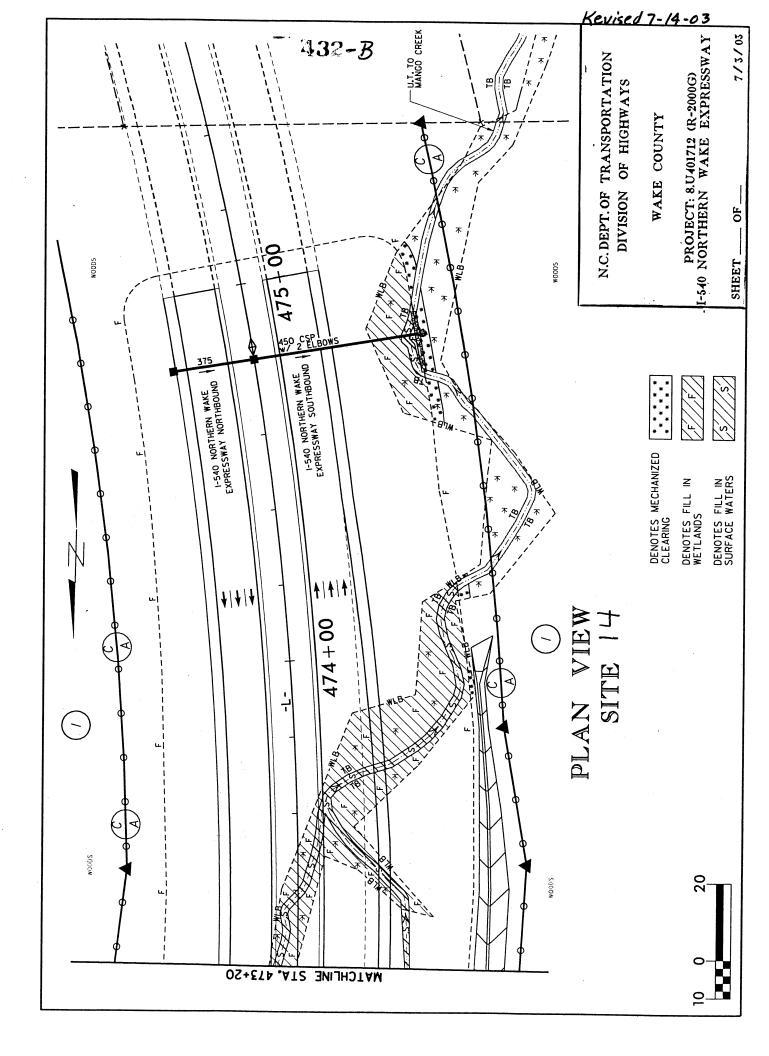
Ms. Kim Canady

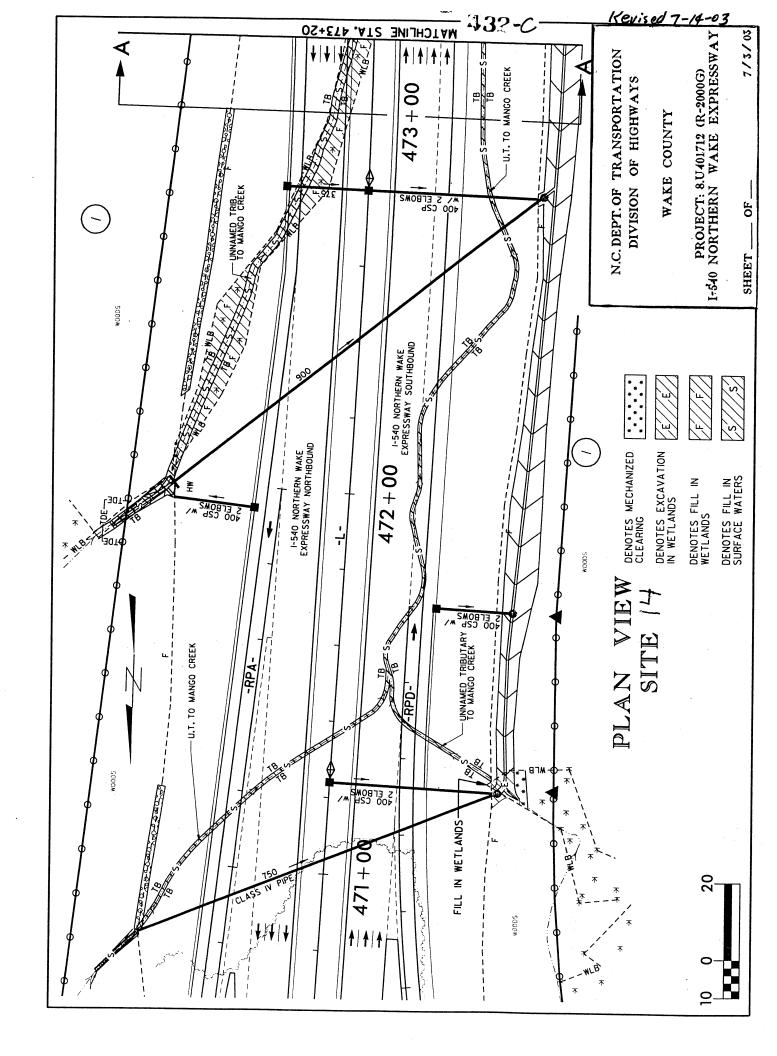
Ms. Yang Steelman

Project File (2)





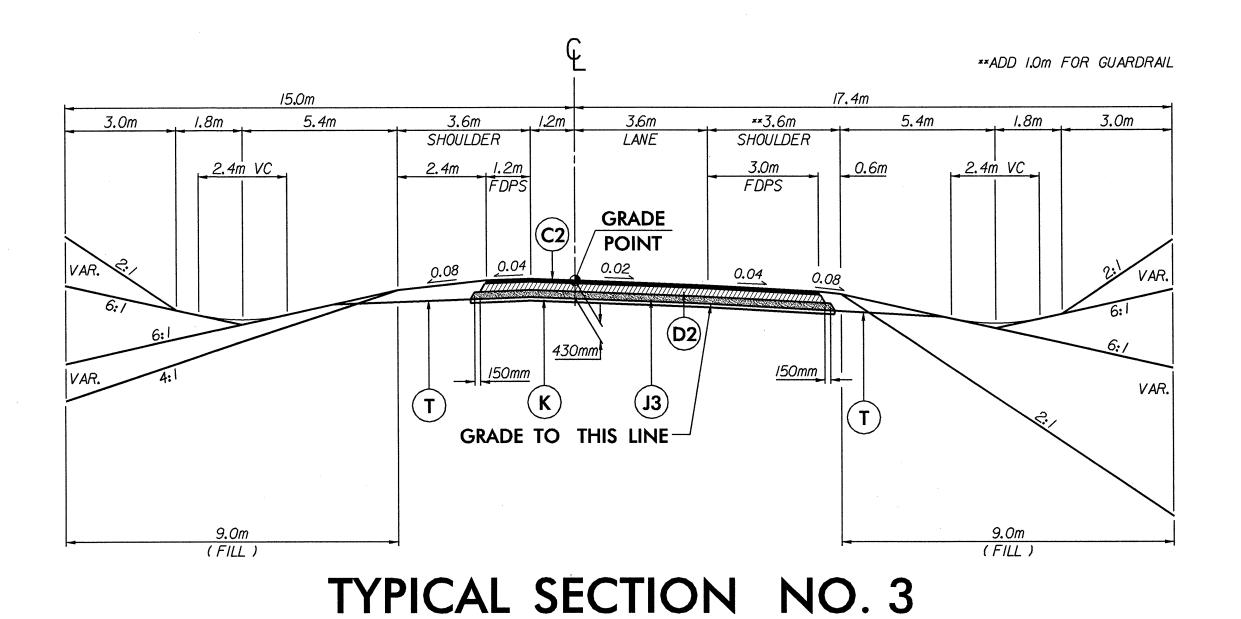




473

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										SHEET	OF					7/3/03

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Ramps

# USE TYPICAL SECTION NO. 3 AT THE FOLLOWING LOCATIONS:

RAMP A -Y5- STA. 1+59.635 TO STA. 6+10.906 RAMP D -Y5- STA. 1+17.764 TO STA. 5+49.688 RAMP B -Y5- STA. 2+21.548 TO STA. 6+03.430

SEE PLANS FOR LOCATION OF TURNING LANES AND TAPERS

\*VARIES FROM 3.6m TO 6.146m FROM STA.3+21.854 TO STA.3+61.854 TO FACE GRADE POINT R1 150mm - GRADE TO THIS LINE

TYPICAL SECTION NO. 4

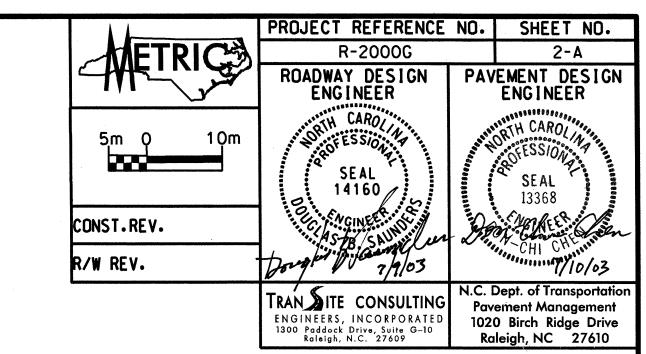
Loop D

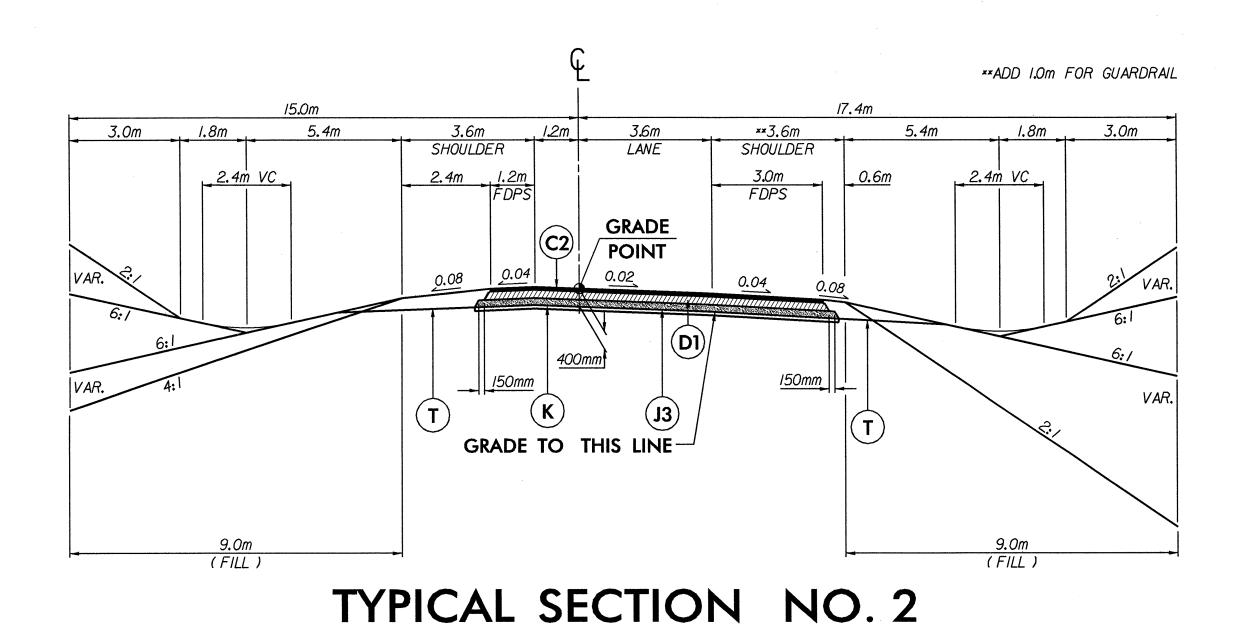
**C2** 70mm \$12.5C D1 80mm 119.0C **D2** 110mm 119.0C **J3** 250mm ABC J4 VAR. DEPTH ABC K SUBGRADE STABILIZATION **R1** 750mm C & G T EARTH MATERIAL

USE TYPICAL SECTION NO. 4 AT THE FOLLOWING LOCATIONS: LOOP D -Y5- STA. 0+62.379 TO STA. 3+61.854

SEE PLANS FOR LOCATION OF TURNING LANES AND TAPERS

ALL PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE





Ramps

USE TYPICAL SECTION NO. 2 AT THE FOLLOWING LOCATIONS:

RAMP C -Y5- STA. 1+45.560 TO STA. 6+69.315

SEE PLANS FOR LOCATION OF TURNING LANES AND TAPERS

C2 70mm S12.5C

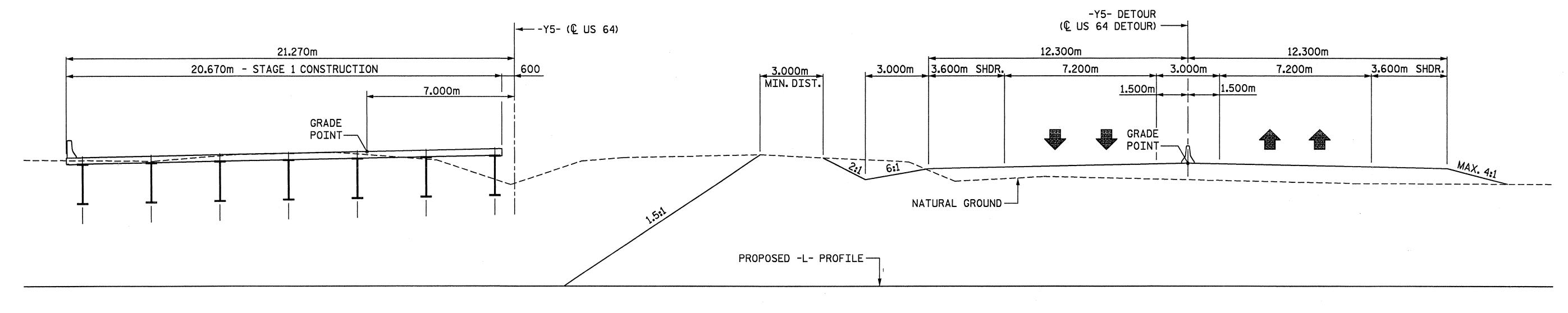
D1 80mm I19.0C

J3 250mm ABC

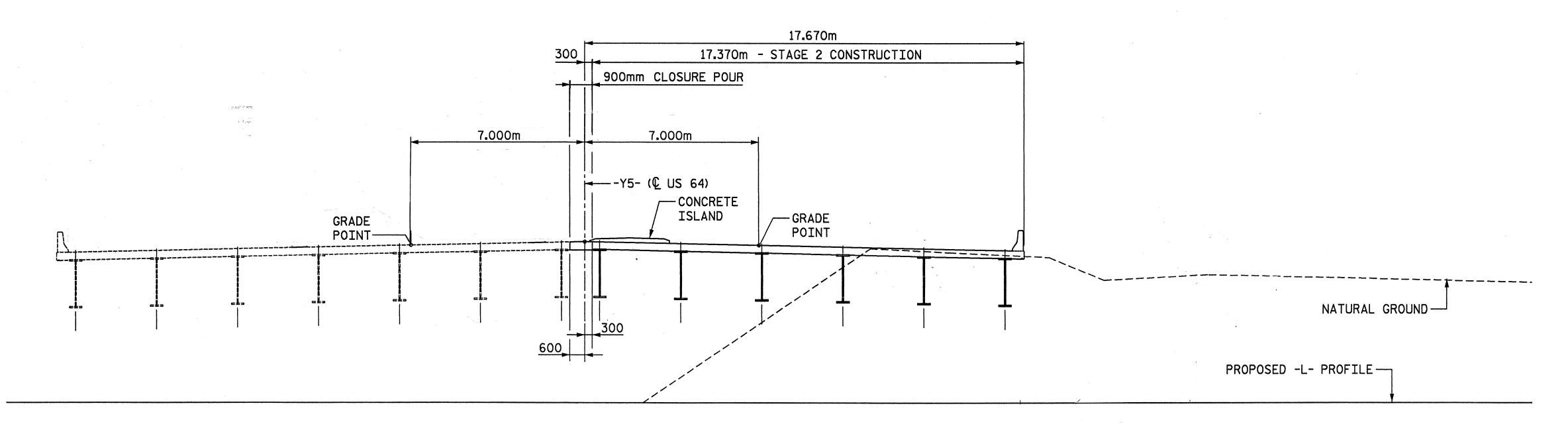
K SUBGRADE STABILIZATION

T EARTH MATERIAL

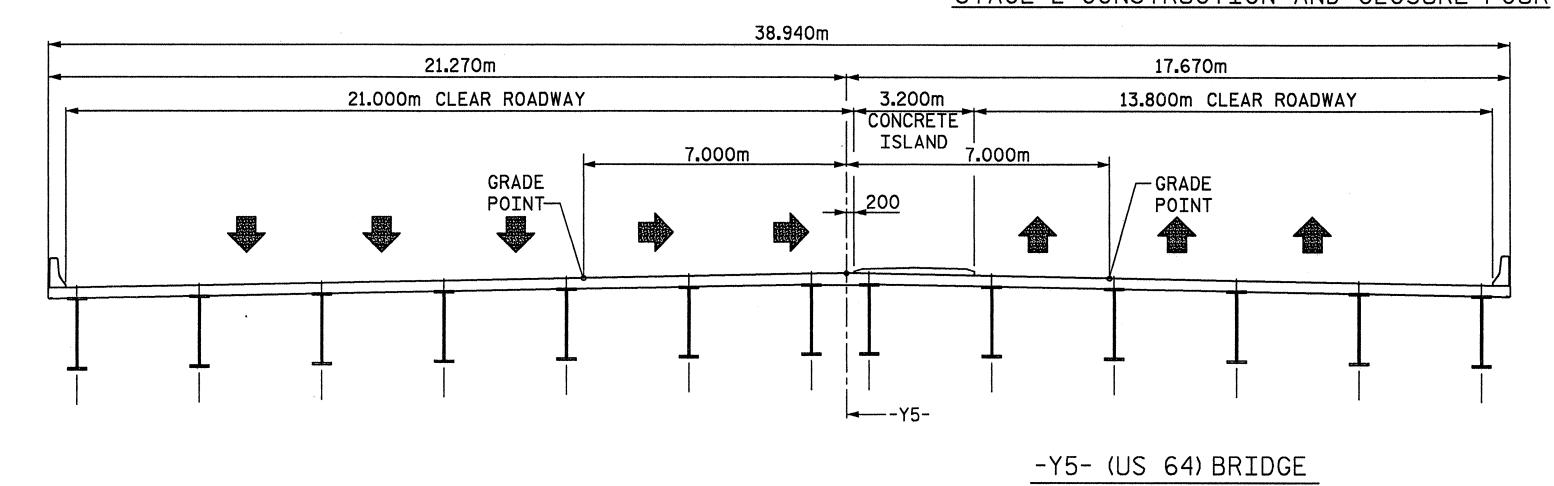
ALL PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE



# STAGE 1 CONSTRUCTION



# STAGE 2 CONSTRUCTION AND CLOSURE POUR



WETRICO.

PROJECT NO. R-2000G

WAKE COUNTY

STATION: 465+66.727 -L- POT

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

CONSTRUCTION SEQUENCE FOR BRIDGE ON US 64 OVER I-540 RALEIGH OUTER LOOP

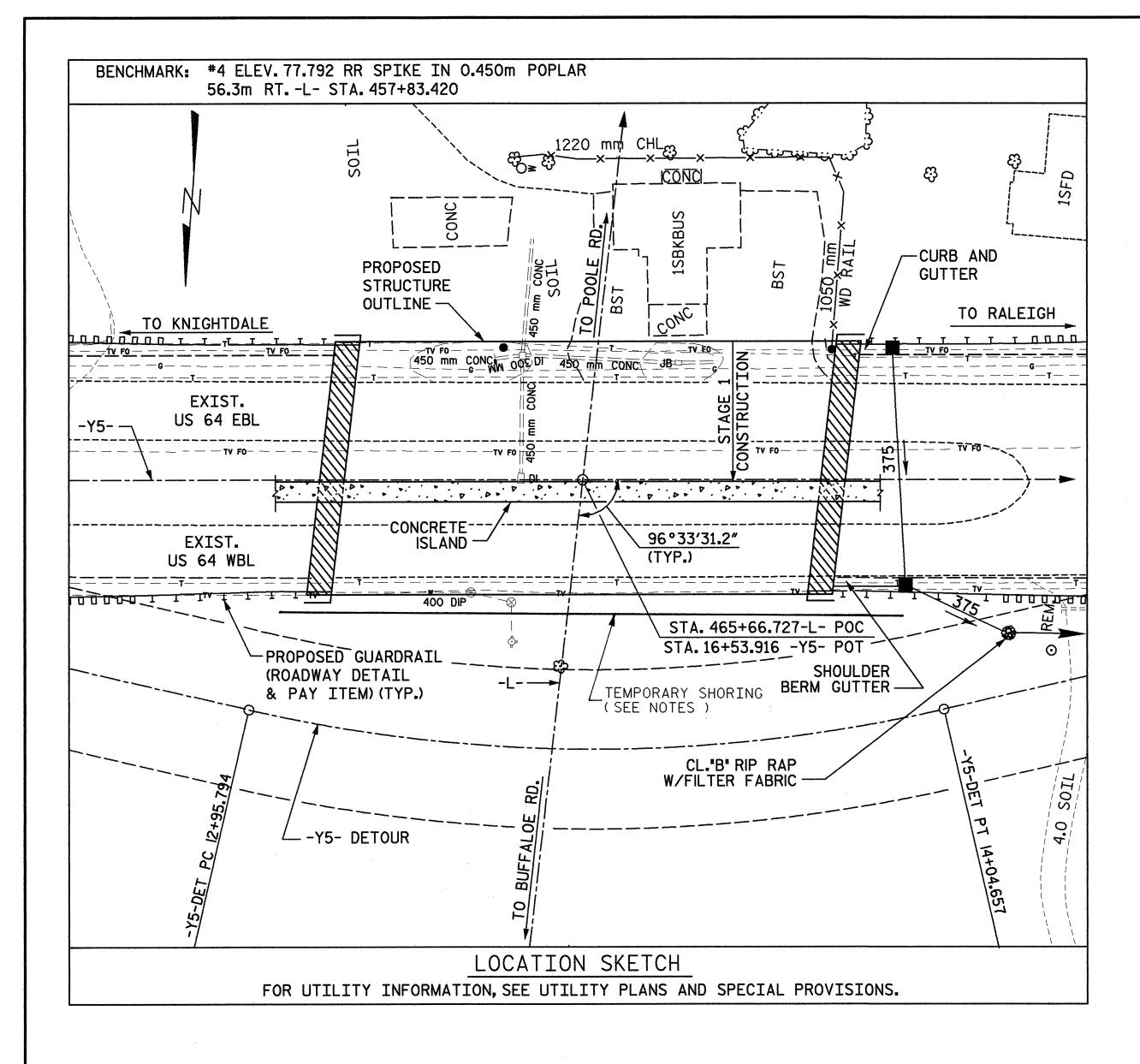
Naliamala & Wilson, P.A.

1399 Ashleybrook Lane, Suite 130
Winston-Salem, N.C. 27103-2918
Ph: (336) 765-4651 Fax: (336) 765-6194

DWG. NO. 2101(3) - 4

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	REV	SIONS		SHEET NO.
BY:	DATE	NO. BY:	DATEs	S-108
		3	·	IOIA SHEETS
		4		45

DRAWN BY: A.S. CALLAWAY. DATE: 07/26/01 CHECKED BY: Hether Welon DATE: 1-29-02



NOTES

ASSUMED LIVE LOAD = MS18 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.

ALL ELEVATIONS ARE IN METERS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SNSM.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 360,000 kg OF REINFORCING STEEL, ONE 760mm SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 360,000 kg OF REINFORCING STEEL, TWO 760mm SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 345W AND PAINTED IN ACCORDANCE WITH SYSTEM 4 OF ARTICLE 442-7 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

PILES FOR END BENTS NO.1 & 2 SHALL BE DRIVEN TO A MINIMUM BEARING CAPACITY OF 530 kN EACH.

THE REQUIRED BEARING CAPACITY OF THE SPREAD FOOTINGS AT BENT NO. 1 IS 380 kPg. THE REQUIRED BEARING CAPACITY SHALL BE VERIFIED.

WORK SHALL NOT BE STARTED ON THIS BRIDGE, AT EACH STAGE OF CONSTRUCTION, UNTIL ROADWAY SECTION IN THAT STAGE HAS BEEN EXCAVATED.

WAITING PERIOD FOR THE APPROACH SLAB CONSTRUCTION SHALL BE WAIVED.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR METRIC STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

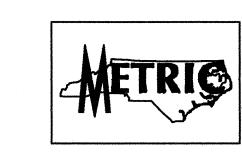
FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

STEEL SHEET PILING REQUIRED FOR SHORING SHALL BE HOT ROLLED.

						TOTAL BILL	OF MATERIA	<b>A</b> L							
	FOUNDATION EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	STRUCTURAL STEEL	i	HP310×79 TEEL PILES	CONCRETE BARRIER RAIL	100mm SLOPE PROTECTION	TFE EXP. BEARING/ ASSEMBLIES	ELASTOMERIC BEARINGS	EVAZOTE JOINT SEALS
	LUMP SUM	SQ. METERS	SQ. METERS	CU. METERS	LUMP SUM	Kg	Kg	APPROX. Kg	NO.	LIN. METERS	LIN. METERS	SQ. METERS	LUMP SUM	LUMP SUM	LUMP SUM
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TOTAL	LUMP SUM	2,905.5	2,444.9	392.0	LUMP SUM	35,492	1,692	389,900	68	408	149.228	930	LUMP SUM	LUMP SUM	LUMP SUM



PROJECT NO. R-2000G

WAKE COUNTY

STATION: 465+66.727 -L- POT

SHEET 3 OF 3

SEAL WILLIAM 2.26-0

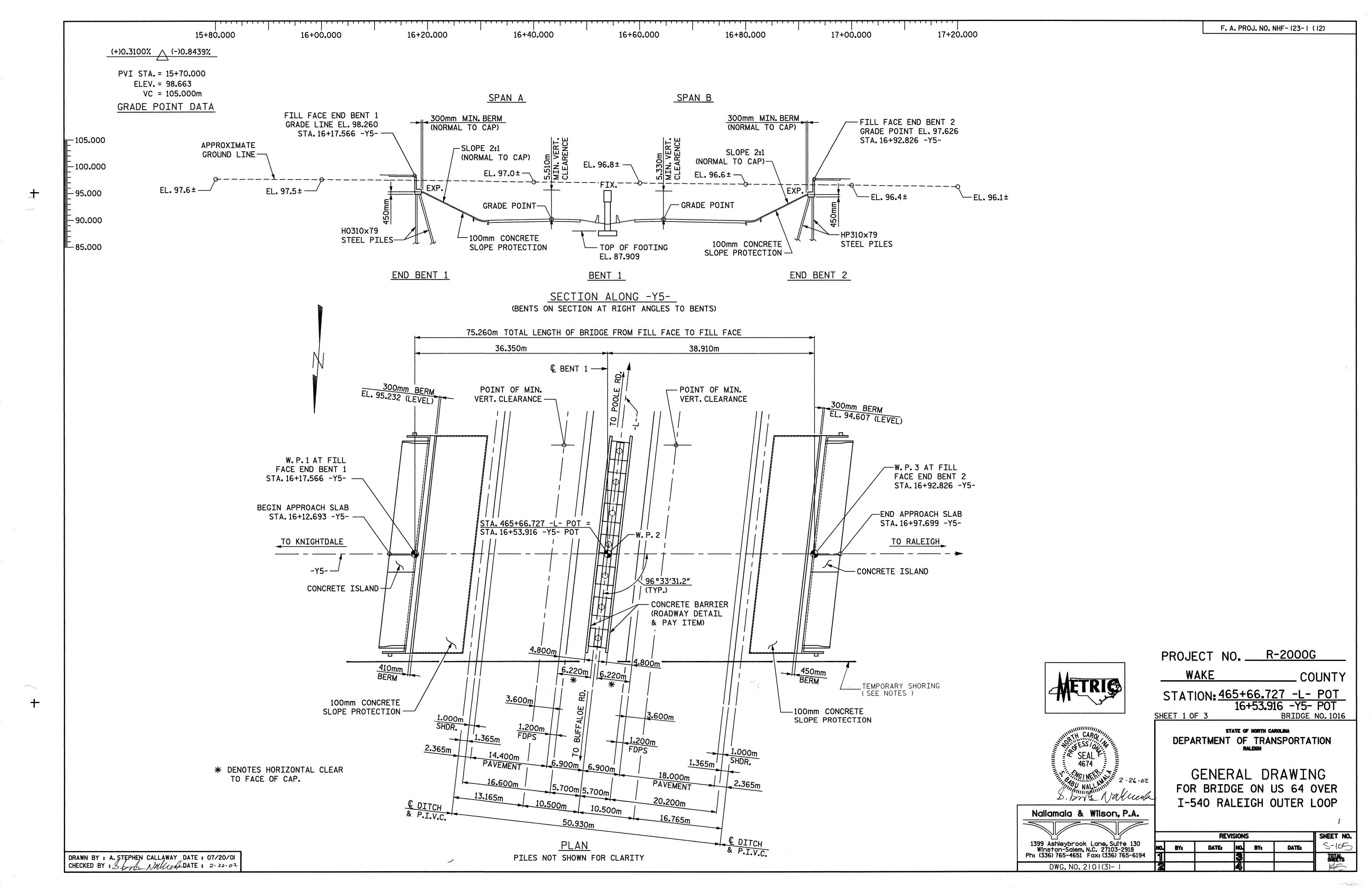
DWG. NO. 2101(3)-3

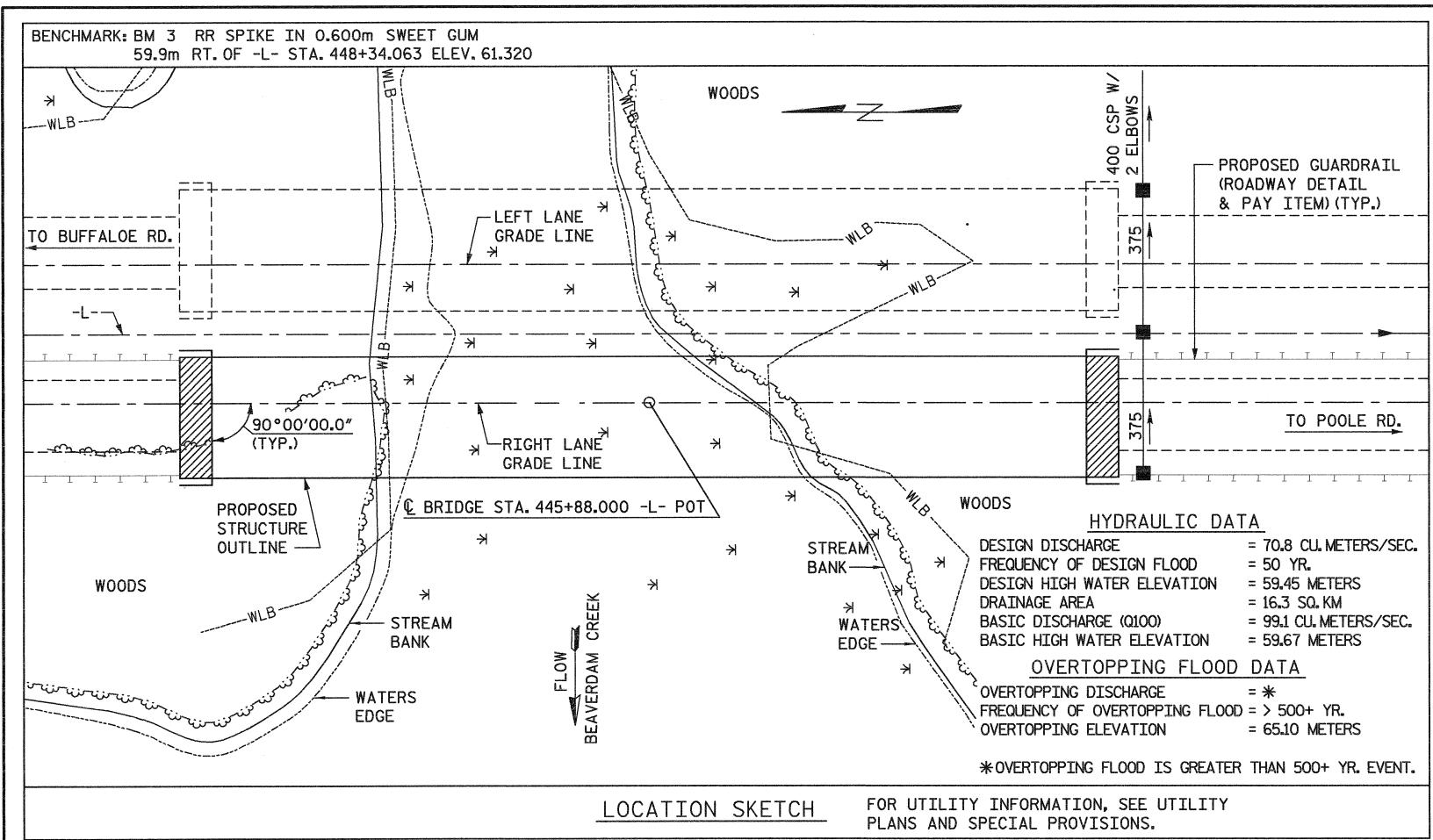
GENERAL DRAWING FOR BRIDGE ON US-64 OVER

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DRAWN BY: A. STEPHEN CALLAWAY DATE: 07/20/01 CHECKED BY: S. Dors A Milliagh DATE: 2-22-02





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	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	1372mm DIA. DRILLED PIERS NOT IN SOIL	1372mm DIA. DRILLED PIERS IN SOIL	PERMANENT STEEL CASING FOR 1372mm DIA. DRILLED PIERS	CROSSHOLE SONIC LOGGING	CSL TUBES	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS
	LUMP SUM	LIN. METERS	LIN. METERS	LIN. METERS	EACH	LIN. METERS	SQ. METERS	SQ. METERS	CU. METERS	LUMP SUM
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TOTAL	LUMP SUM	34.5	74.7	42.4	3	473.4	2,418.7	2,331.6	316.5	LUMP SUM

				TOTAL E	3ILL	OF MATER	RIAL					NO HIGH
	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	P	DIFIED 1829mm RESTRESSED CRETE GIRDERS		P360x108 EEL PILES	CONCRETE BARRIER RAIL	PLAIN RIP RAP CLASS II (600mm THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	EXPANSION JOINT SEALS	SHAFT 3 TIP BEA DRILLED HIGHER TIP BEA
	Kg	Kg	NO.	LIN. METERS	NO.	LIN. METERS	LIN. METERS	METRIC TONS	SQ. METERS	LUMP SUM	LUMP SUM	DRILLED
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BENT 2	11,563	3,396				Authorização de companya de co	abicomonicamente de la constitución de la constituc	OR ANNIAN AND AND AND AND AND AND AND AND AND A	gandermon production and production and an according to the contract of the co	and Albumbur contact of the Contact	epica di Digerra productivo del considera per di signi di seriesi di signi di seriesi di signi di seriesi di signi di seriesi di ser	ELECTRICA
BENT 3	10,773	3,043	***************************************		aur.incontitional/emission			-	Maria decenti y di mananta del mananta			CONDUIT SYSTEM
END BENT 2	4,591				20	250.0		755	770			
TOTAL	41,316	9,058	24	785.760	40	390.0	263.760	1,170	1,190	LUMP SUM	LUMP SUM	LUMP SUM

NOTES

ASSUMED LIVE LOAD = MS18 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.

ALL ELEVATIONS ARE IN METERS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SNSM.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 360,000 kg OF REINFORCING STEEL, ONE 760mm SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 360,000 kg OF REINFORCING STEEL, TWO 760mm SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED CAPACITY OF THE DRILLED PIERS AT BENTS NO. 1, 2 AND 3. WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE. PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", NOVEMBER, 1995.

FOR METRIC STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

THE DRILLED PIERS FOR BENTS NO. 1, 2 AND 3 HAVE BEEN DESIGNED FOR BOTH SKIN FRICTION AND TIP BEARING. THE REQUIRED TIP BEARING CAPACITY IS 1915 kPa.

THE REQUIRED TIP BEARING CAPACITY AT BENTS NO. 1, 2 AND 3 SHALL BE VERIFIED.

THE DRILLED PIERS FOR BENTS NO. 1, 2 AND 3 HAVE BEEN DESIGNED FOR AN APPLIED LOAD OF 2.510 KN EACH AT THE TOP OF THE COLUMN.

PERMANENT STEEL CASING IS REQUIRED FOR THE DRILLED PIERS AT BENT NO.1 AND THE CASINGS SHALL NOT EXTEND BELOW ELEVATION 53.0m WITHOUT THE ENGINEER'S PERMISSION.

PERMANENT STEEL CASING IS REQUIRED FOR THE DRILLED PIERS AT BENT NO. 2 AND THE CASINGS SHALL NOT EXTEND BELOW ELEVATION 53.5m WITHOUT THE ENGINEER'S PERMISSION.

PERMANENT STEEL CASING IS NOT REQUIRED FOR THE DRILLED PIERS AT BENT NO. 3.

FOR PERMANENT STEEL CASING, SEE SPECIAL PROVISIONS FOR DRILLED PIERS.

DRILLED PIERS AT BENT NO.1 SHALL EXTEND TO ELEVATIONS NO HIGHER THAN THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 52.0m, SHAFT 2 = 52.0m, SHAFT 3 = 50.0m, SHAFT 4 = 50.0m AND SATISFY THE REQUIRED TIP BEARING CAPACITY.

DRILLED PIERS AT BENT 2 SHALL EXTEND TO AN ELEVATION NO HIGHER THAN 47.5m FOR ALL SHAFTS AND SATISFY THE REQUIRED TIP BEARING CAPACITY.

DRILLED PIERS AT BENT 3 SHALL EXTEND TO ELEVATIONS NO HIGHER THAN THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 53.0m, SHAFT 2 = 52.0m, SHAFT 3 = 50.0m, SHAFT 4 = 47.0m AND SATISFY THE REQUIRED TIP BEARING CAPACITY.



DWG. NO. 2101(2)RS-3

THE SCOUR CRITICAL ELEVATIONS FOR BENT 1 ARE 53.0. THESE ELEVATIONS ARE FOR USE BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATIONS FOR BENT 2 ARE THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 49.5m, SHAFT 2 = 50.5m, SHAFT 3 = 51.5m AND SHAFT 4 = 52.5m. THE SCOUR CRITICAL ELEVATIONS ARE USED BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATIONS FOR BENT 3 ARE THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 55.0m, SHAFT 2 = 54.0m, SHAFT 3 = 53.0m AND SHAFT 4 = 52.0m. THE SCOUR CRITICAL ELEVATIONS ARE USED BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

FOR DRILLED PIERS, SEE SPECIAL PROVISIONS.

SPT TESTING IS NOT REQUIRED TO DETERMINE THE TIP BEARING

SLURRY CONSTRUCTION SHALL NOT BE USED FOR THIS PROJECT.

SID INSPECTIONS ARE NOT REQUIRED TO DETERMINE THE BOTTOM CLEANLINESS OF THE DRILLED PIERS AT BENTS NO. 1. 2 AND 3.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS AT BENTS NO. 1, 2 AND 3. SEE SPECIAL PROVISIONS FOR CROSSHOLE SONIC LOGGING.

THE CONTRACTOR SHALL OBSERVE A ONE-MONTH WAITING PERIOD BEFORE BEGINNING ANY WORK FOR END BENT CONSTRUCTION AFTER COMPLETION OF THE EMBANKMENT AT EACH END BENT. THE CONTRACTOR MAY BEGIN THE REINFORCED BRIDGE APPROACH FILL CONSTRUCTION AFTER COMPLETION OF END BENT INCLUDING WINGWALLS.

PILES FOR END BENTS NO.1 AND 2 SHALL BE DRIVEN TO A MINIMUM BEARING CAPACITY OF 620 KN EACH

WHEN DRIVING PILES. THE MAXIMUM BLOW COUNT SHALL NOT BE EXCEEDED.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR WORK BRIDGE, SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS. FOR PAY ITEM FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS. SEE LEFT LANE.

FOR ELECTRICAL CONDUIT SYSTEM, SEE SPECIAL PROVISIONS

R-2000G PROJECT NO. \_\_ WAKE COUNTY STATION: 445+88.000 -L- POT

SHEET 3 OF 3

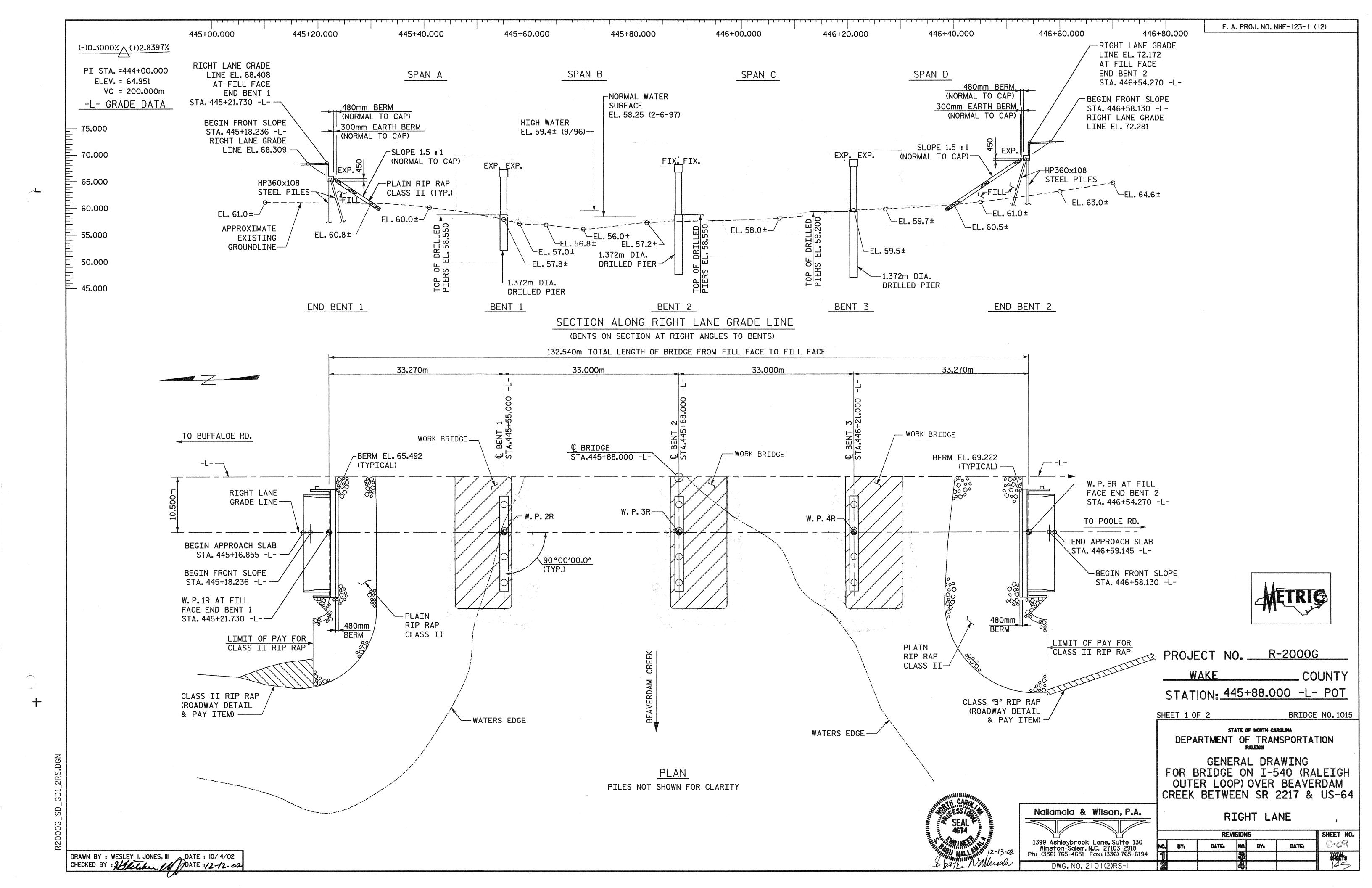
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

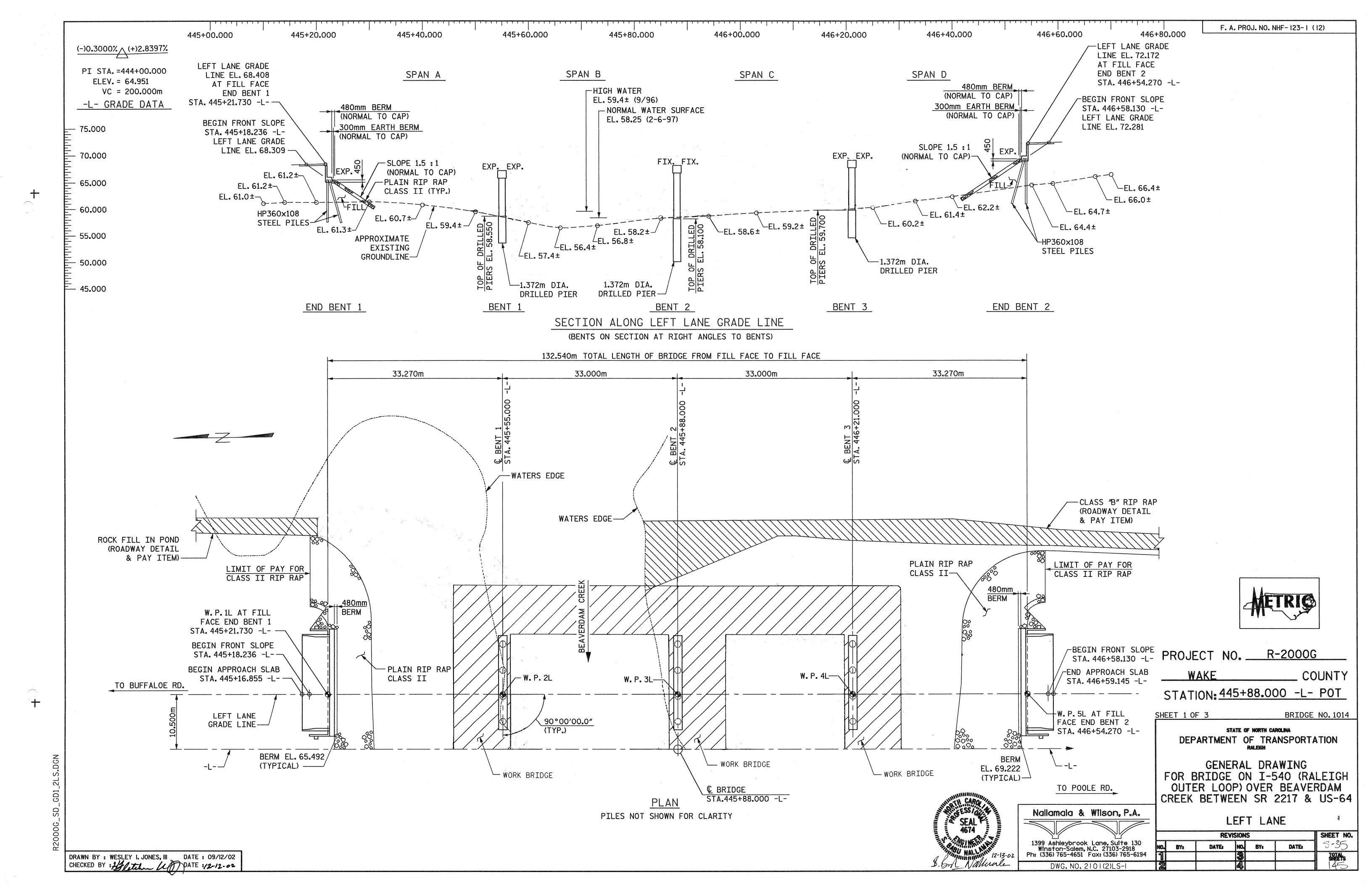
GENERAL DRAWING FOR BRIDGE ON I-540 (RALEIGH OUTER LOOP) OVER BEAVERDAM CREEK BETWEEN SR 2217 & US-64

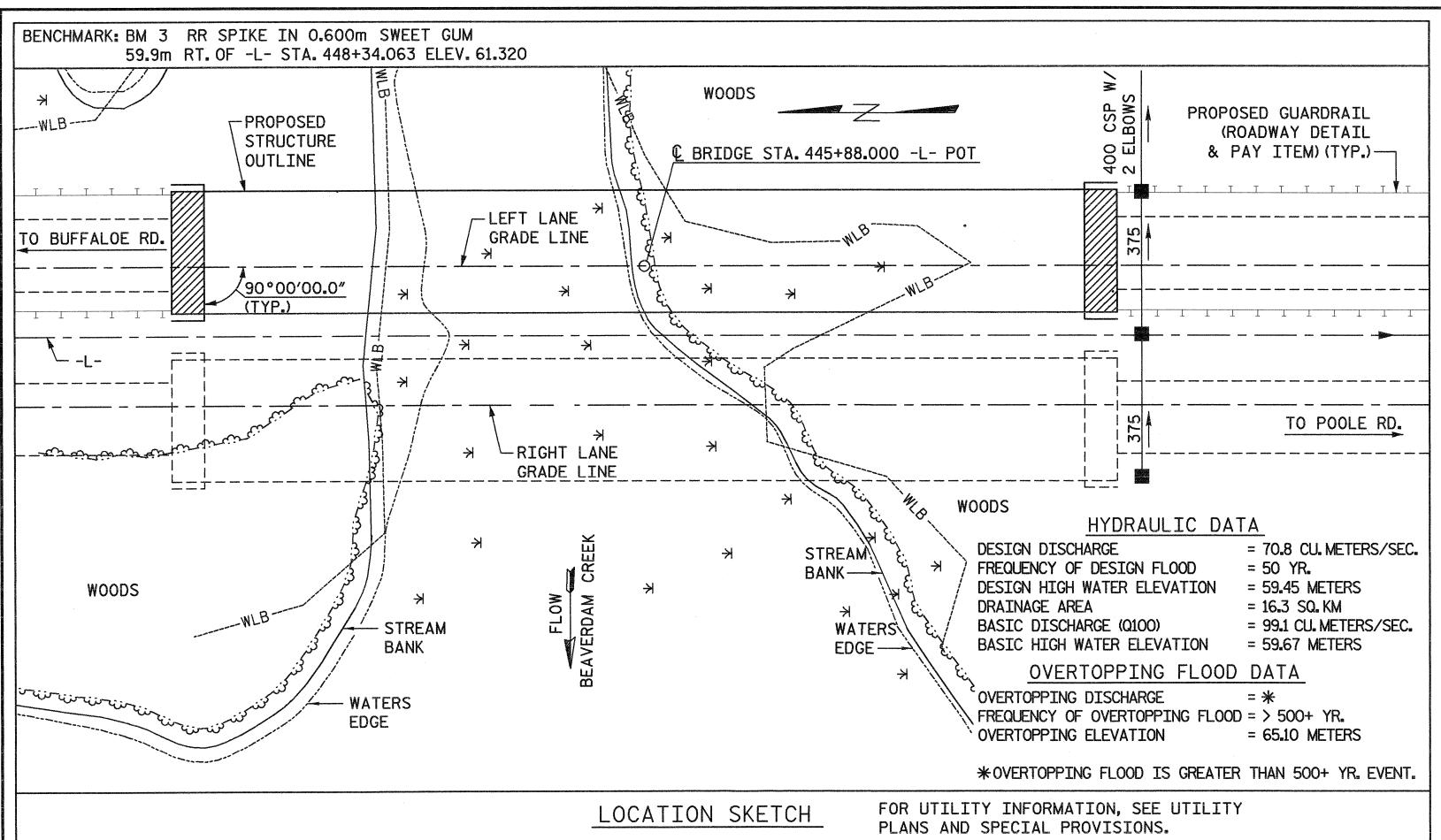
RIGHT LANE

SHEET NO. REVISIONS 8-71 NO. BY: BYs DATES DATES

DRAWN BY: WESLEY I. JONES, III DATE: 10/14/02 CHECKED BY: Affilia LAP DATE: 12-12-02







		·	TOTAL B	SILL OF MATE	ERIAL					
	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	· ·	1372mm DIA. DRILLED PIERS IN SOIL	PERMANENT STEEL CASING FOR 1372mm DIA. DRILLED PIERS	CROSSHOLE SONIC LOGGING	CSL TUBES	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS
	LUMP SUM	LIN. METERS	LIN. METERS	LIN. METERS	EACH	LIN. METERS	SQ. METERS	SQ. METERS	CU. METERS	LUMP SUM
SUPERSTRUCTURE		4-11-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	***************************************				2,418.7	2.331.6		
END BENT 1						BEQUIRES AND PROPERTY OF THE P		CALL COMPANIES AND ADDRESS AND	47.5	LUMP SUM
BENT 1	LUMP SUM	11.0	11.2	9.2	1	101.0	contraction of the contraction o		70.0	от поставления в поста
BENT 2	LUMP SUM	12.0	21.4	15.9	1	145.8			76.5	
BENT 3	LUMP SUM	13.0	7.8	and the second s	1	95.4			73.4	
END BENT 2						generation from the opposition of control may be a finished and the control of th	финасын саймасанда откай я этом сада об об об основа сада об		48.8	LUMP SUM
TOTAL	LUMP SUM	36.0	40.4	25.1	3	342.2	2,418.7	2,331.6	316.2	LUMP SUM

				TOTAL	BIL	OF MATE	RIAL				
	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PF	DIFIED 1829mm RESTRESSED RETE GIRDERS	l	2360×108 EL PILES	CONCRETE BARRIER RAIL	PLAIN RIP RAP CLASS II (600mm THICK)	FILTER FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	EXPANSION JOINT SEALS
	Kg	Kg	NO.	LIN. METERS	NO.	LIN. METERS	LIN. METERS	METRIC TONS	SQ. METERS	LUMP SUM	LUMP SUM
SUPERSTRUCTURE	and a second and a		24	785.760			263.760			LUMP SUM	LUMP SUM
END BENT 1	4,512	May se of Construction and the Assessment			20	150.0	40000000000000000000000000000000000000	350	360	mental and dispulse construction of the delicities comp	посметний поститути почини и подмератили почини подмер
BENT 1	9,118	2,274	Quantificação (institutivos	estatory popularity autopour recombination (1999)	OLOGOVERSON PROTOSTO	Material Section - Color Addition of Section Addition additio			MANAPARINALISMA SERVICE SERVIC		anne de Carlo de Car
BENT 2	10,708	3,013	CALL SCALAR AND		ectopoperasinosimotorina	elon in consequencia de la conse	Application benefit to the control of the control o	And any process of the second and an advantage of the second and an advantage of the second and an advantage of the second and advantage of th	sadenjeouroncommunikov totkiskinski reja		
BENT 3	9,255	2,347	Manifestration	Albania marini di Aringilindi di Agli di Banashi ndan adare	and the section of th	Anchorate representative to the Parket Representative Co.	and a million de Made Made Made and a state of the second	dabitasis numusus miduud na dacumarium turad	National contraction of the second of the se	ACC illustration and production and	
END BENT 2	4,591	spine country of the country in the country of the	Constraint distribution of the constraint of the		20	160.0		410	415	ediscontinue dischilinistic provinci qui au si colore	
TOTAL	38,184	7,634	24	785.760	40	310.0	263.760	760	775	LUMP SUM	LUMP SUM

# NOTES

ASSUMED LIVE LOAD = MS18 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC PERFORMANCE CATEGORY A.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE NOTED.

ALL ELEVATIONS ARE IN METERS.

THIS BRIDGE HAS BEEN DESIGNED BY THE STRENGTH DESIGN METHOD AS SPECIFIED IN AASHTO STANDARD SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SNSM.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 360,000 kg OF REINFORCING STEEL, ONE 760mm SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 360,000 kg OF REINFORCING STEEL. TWO 760mm SAMPLES OF EACH SIZE BAR USED. THE SCOUR CRITICAL ELEVATIONS FOR BENT NO. 3 ARE 56.5. THESE THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED ELEVATIONS ARE USED BY MAINTENANCE FORCES TO MONITOR WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE. PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION SLURRY CONSTRUCTION SHALL NOT BE USED FOR THIS PROJECT. AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", NOVEMBER, 1995.

FOR METRIC STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

THE DRILLED PIERS FOR BENTS NO. 1, 2 AND 3 HAVE BEEN DESIGNED FOR BOTH SKIN FRICTION AND TIP BEARING. THE REQUIRED TIP BEARING CAPACITY IS 1915 kPa.

THE REQUIRED TIP BEARING CAPACITY AT BENTS NO. 1, 2 AND 3 SHALL BE VERIFIED.

THE DRILLED PIERS FOR BENTS NO. 1, 2 AND 3 HAVE BEEN DESIGNED FOR AN APPLIED LOAD OF 2.510 KN EACH AT THE TOP OF THE COLUMN.

PERMANENT STEEL CASING IS REQUIRED FOR THE DRILLED PIERS AT BENT NO. 1 AND THE CASINGS SHALL NOT EXTEND BELOW THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 56.5m, SHAFT 2 = 56.5m, SHAFT 3 = 56.5m, SHAFT 4 = 55.5m WITHOUT THE ENGINEER'S PERMISSION.

PERMANENT STEEL CASING IS REQUIRED FOR THE DRILLED PIERS AT BENT NO. 2 AND THE CASINGS SHALL NOT EXTEND BELOW THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 54.5m, SHAFT 2 = 54.5m, SHAFT 3 = 55.0m, SHAFT 4 = 52.5m WITHOUT THE ENGINEER'S PERMISSION.

PERMANENT STEEL CASING IS NOT REQUIRED FOR THE DRILLED PIERS AT BENT NO. 3.

FOR PERMANENT STEEL CASING, SEE SPECIAL PROVISIONS FOR DRILLED PIERS.

DRILLED PIERS AT BENT NO. 1 SHALL EXTEND TO ELEVATIONS NO HIGHER THAN THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 53.5m, SHAFT 2 = 53.5m, SHAFT 3 = 53.5m. SHAFT 4 = 51.5m AND SATISFY THE REQUIRED TIP BEARING CAPACITY.

DRILLED PIERS AT BENT 2 SHALL EXTEND TO ELEVATIONS NO HIGHER THAN THE FOLLOWING LISTED ELEVATIONS FROM LEFT TO RIGHT: SHAFT 1 = 51.0m, SHAFT 2 = 51.0m, SHAFT 3 = 49.0m. SHAFT 4 = 48.0m AND SATISFY THE REQUIRED TIP BEARING CAPACITY.

DRILLED PIERS AT BENT 3 SHALL EXTEND TO AN ELEVATION NO HIGHER THAN 54.5m FOR ALL SHAFTS AND SATISFY THE REQUIRED TIP BEARING CAPACITY.

THE SCOUR CRITICAL ELEVATION FOR BENT NO. 1 IS 55.0. THESE ELEVATIONS ARE FOR USE BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATIONS FOR BENT NO. 2 ARE 53.0. THESE ELEVATIONS ARE USED BY MAINTENANCE FORCES TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

FOR DRILLED PIERS, SEE SPECIAL PROVISIONS.

SPT TESTING IS NOT REQUIRED TO DETERMINE THE TIP BEARING CAPACITY OF THE DRILLED PIERS AT BENTS NO. 1. 2 AND 3.

SID INSPECTIONS ARE NOT REQUIRED TO DETERMINE THE BOTTOM CLEANLINESS OF THE DRILLED PIERS AT BENTS NO. 1, 2 AND 3.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS AT BENTS NO. 1, 2 AND 3. SEE SPECIAL PROVISIONS FOR CROSSHOLE SONIC LOGGING.

THE CONTRACTOR SHALL OBSERVE A ONE-MONTH WAITING PERIOD BEFORE BEGINNING ANY WORK FOR END BENT CONSTRUCTION AFTER COMPLETION OF THE EMBANKMENT AT EACH END BENT. THE CONTRACTOR MAY BEGIN THE REINFORCED BRIDGE APPROACH FILL CONSTRUCTION AFTER COMPLETION OF END BENT INCLUDING WINGWALLS.

PILES FOR END BENTS NO. 1 AND 2 SHALL BE DRIVEN TO A MINIMUM BEARING CAPACITY OF 620 KN EACH.

WHEN DRIVING PILES, THE MAXIMUM BLOW COUNT SHALL NOT BE EXCEEDED.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR WORK BRIDGE, SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS.

SHEET 3 OF 3

PROJECT NO. R-2000G WAKE COUNTY STATION: 445+88.000 -L- POT

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

Nallamala & Wilson, P.A.

1399 Ashleybrook Lane, Suite 130 Winston-Salem, N.C. 27103-2918 Ph: (336) 765-4651 Fax: (336) 765-6194 DWG. NO. 2101(2)LS-3

GENERAL DRAWING FOR BRIDGE ON I-540 (RALEIGH OUTER LOOP) OVER BEAVERDAM

LEFT LANE

CREEK BETWEEN SR 2217 & US-64

REVISIONS SHEET NO. 5-37 NO. BY: DATES DATE BYs

DRAWN BY: WESLEY I. JONES, III DATE: 09/12/02 CHECKED BY: WILLELE LAND DATE: 12-12-02

IUIAL